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DETERMINATION OF LAME'S PARAMETERS (λ and μ) FROM COMPRESSIONAL AND SHEAR WAVE VELOCITY FOR UYO AND ENVIRON, SOUTHEASTERN NIGERIA



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ABSTRACT

In this work, compressional and shear wave velocity values were determined for the estimation of Lamé's first and second constants (λ , μ) for Uyo and environ, which helped in the determination of elastic parameters of top soil as well as the degree of stability of engineering foundations. The study area lies between latitudes 4°45' and 5°15' N and between longitudes 7°45' and 8°30' E in the Niger Delta region of Southern Nigeria. A 24-channel signal enhancement seismograph was used in generating seismic waves. The mechanically generated seismic disturbances sensed by the geophones were received and recorded by a seismograph cascaded with the geophones. Shear modulus (μ) values ranged from 0.21×10^8 to 0.63×10^8 N/m² with an average of 0.43×10^8 N/m² for layer 1 and 0.78×10^8 to 2.55×10^8 N/m² with an average of 1.40×10^8 N/m² for layer 2. Also, λ values ranged from 0.20×10^8 to 0.58×10^8 N/m² with an average of 0.40×10^8 N/m² for layer 1 and 0.73×10^8 to 2.36×10^8 N/m² with an average of 1.30×10^8 N/m² for layer 2. The results indicate that the topsoil under study can support load that is being subjected to shear stress, provided the materials within the layer are well compressed.

KEYWORDS: Uyo, Wave Velocity, Seismic, Layer, Foundation

INTRODUCTION

The Lamé's parameters (λ and μ) are two material-dependent quantities in continuum mechanics which arises in stress-strain relationships. They are sometimes referred to as the Lamé's first parameter (λ) and Lamé's second parameter (μ) (Weisstein, 2015). Kazemi (2012) opined that λ is sensitive to the fluid within the rock fabric, whereas μ is sensitive to the rock matrix. The Lamé's constant (λ) and shear modulus (μ) form the two most important parameters in the study of fluids μ and reservoir rock. The constants λ and μ arise in strain- stress relationships. They are given in terms of other solid properties as:

$$\mu = \frac{E}{2(1+\sigma)} \quad (1)$$

$$\lambda = \frac{\sigma E}{(1+\sigma)(1-2\sigma)} \quad (2)$$

where E is Young's modulus and σ is the Poisson ratio (Akankpo and Essien, 2015).

New understanding into the original rock properties is offered by the conversion of velocity measurements to Lamé's moduli parameters. These parameters allow for enhanced identification of reservoir zones (Ezeh, 2014). Several authors had worked on and provided the theory and concept for extracting Lamé's parameters (Buriyank, 2000; Goodway, 2001. According to Sokolnikov (1972), Lamé's constants (λ , μ) of homogeneous elastic material were always determined using conventional methods like mechanical testing, which was destructive. This led to the

search for an alternative method by measuring the longitudinal and transversal velocity, which is known as Non-Destructive Evaluation (NDE) (Forsythe *et al.*, 1977). In this work, compressional and shear wave velocity values were estimated for the determination of Lamé's parameters for the area. This helped in determining the elastic parameters of top soil and the degree of stability of engineering foundation.

LOCATION AND GEOLOGY OF UYO AND ENVIRON

The study area lies between latitudes 4°45' and 5°15' N and between longitudes 7°45' and 8°30' E in the Niger Delta region of Southern Nigeria. It is situated in an equatorial climatic region, having two major seasons: the rainy season and dry season (Evans *et al.*, 2010). Due to the current global climate changes, there are shifts in both the upper and lower boundaries of these climatic conditions (Farauta, *et al.*, 2012, Akpabio and Ukaegbu, 2009).

Uyo is located in the Benin Formation (Coastal Plain Sands (CPS)) and Alluvium environments of the Niger Delta region of Southern Nigeria (Figure 1). 1. The Formation covers over 80% of the study area and beneath it is the parallel Agbada Formation (Reijers *et al.*, 1997; Nganje *et al.*, 2007). The engineering foundations in the area concentrate within the Benin Formation. The Formation comprises fine-medium-coarse sands and gravels which are poorly sorted (Essien and Akankpo, 2013; Essien, *et al.*, 2014). The area is generally porous, permeable and usually interrupted by clay-sand sequence at various depths (Okwueze, 1991a; Ekweme & Onyeagoda, 1985).

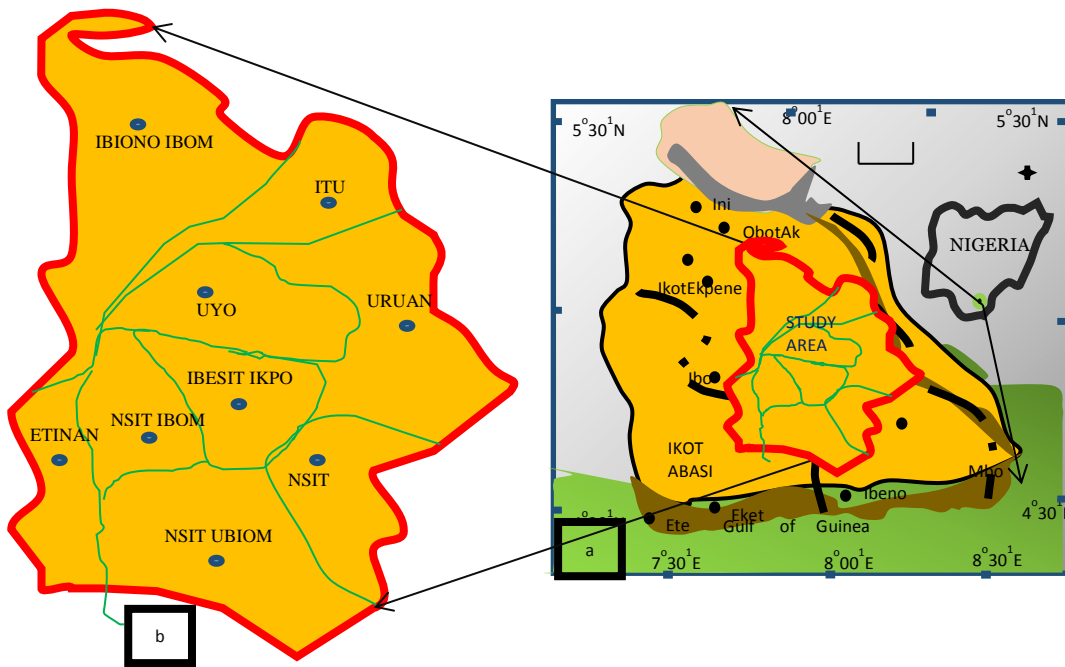


Fig. 1: Map showing (a) the study area location and general geology of Akwa Ibom State of Nigeria (b) Map showing the central Uyo district that the study area situates

MATERIALS AND METHOD

A 24 - channel signal enhancement seismograph was used in generating seismic wave. The electromagnetic geophone which were in direct contact with the earth, transformed the seismic energy generated by the source to electrical voltage which is a function of velocity. The mechanically generated seismic disturbances sensed by the geophones were received and recorded by a seismograph cascaded with the geophones (Kearey and Brooks, 1991; Reynolds, 1997, Akpabio and Adeniran 2023).

The generated energy penetrated into the subsurface and refracted off at various interfaces corresponding to the geological boundaries and consequently returned to the surface at later time to be picked up by the geophone (Kearey and Brooks, 1991). The seismic wave received by the geophone was converted into electrical pulse and was amplified by the preamplifier. The generated waves and energy penetrated into the subsurface and are retracted off at various interfaces corresponding to geological boundaries and consequently returned to the surface at a later time to be picked up by the geophones (Kumar and Kumar, 2003, Obianwu, et al, 2019).

RESULTS AND DISCUSSION

The values of the first and second Lamé's constants are shown in Tables 1 and 2. Shear modulus (μ) values ranged from 0.21×10^8 to 0.63×10^8 N/m² with an average of 0.43×10^8 N/m² for layer 1 and 0.78×10^8 to 2.55×10^8 N/m²

with an average of 1.40×10^8 N/m² for layer 2. Also, λ values ranged from 0.20×10^8 to 0.58×10^8 N/m² with an average of 0.40×10^8 N/m² for layer 1 and 0.73×10^8 to 2.36×10^8 N/m² with an average of 1.30×10^8 N/m² for layer 2. The values of Lamé's constants (μ , λ) in this work testify that the bearing pressure of the foundation layers is nearly isotropic and consequently not susceptible to deformation. Lamé's first constant and shear modulus (the second Lamé's constant) are linearly related in this study (Tables 1 and 2). Using 3D contour maps in Figures 2(a, b) and 3(a, b), the distributions of the first and second Lamé's parameters in the study area were examined.

The geoelectric parameters of the engineering foundation fall within dense sands and gravels as well as silty sands (Sawangsurriya 2012). Soils with high arenaceous formations have a higher bearing capacity than soil with high argillaceous materials (Atat et al., 2013). The range indicates that the topsoil under study can support load that is being subjected to shear stress, provided the materials within the layer are well compressed. The considered foundation layers are cohesionless, gritty and therefore not susceptible to creep, erosion and failures provided proper compaction is done during road construction. The Ultimate Bearing Capacity depends on the soil type, moisture content, compaction and the amount of uniformity of the formation. The higher values of these constants increase the cohesion of the topsoil.

Table 1: Summary of Lamé's Parameters (μ , λ)

| Location Name | Number | Latitude (°) | Longitude (°) | Elevation (m) | Layer | V _p (m/s) | V _s (m/s) | $\mu \times 10^8$ (N/m ²) | $\lambda \times 10^8$ (N/m ²) |
|---------------|-----------|--------------|---------------|---------------|-------|----------------------|----------------------|---------------------------------------|---|
| Etinan | 1 | 5.9833 | 7.8500 | 67.00 | L1 | 285.0 | 128.8 | 0.37 | 0.34 |
| | 2 | 4.9500 | 7.8333 | 61.00 | L2 | 556.9 | 252.9 | 1.41 | 1.30 |
| | | | | | L1 | 327.5 | 148.2 | 0.48 | 0.45 |
| | 3 | 4.8333 | 7.8510 | 31.00 | L2 | 503.4 | 228.5 | 1.15 | 1.06 |
| | | | | | L1 | 317.0 | 143.4 | 0.45 | 0.42 |
| | Nsit Ibom | 1 | 4.8166 | 7.8330 | 36.00 | L1 | 350.0 | 158.5 | 0.55 |
| 2 | | 4.8667 | 7.9167 | 46.00 | L2 | 656.3 | 298.3 | 1.96 | 1.81 |
| | | | | | L1 | 350.0 | 158.5 | 0.55 | 0.51 |
| 3 | | 4.8510 | 7.9000 | 43.00 | L2 | 603.7 | 274.3 | 1.66 | 1.53 |
| | | | | | L1 | 291.0 | 131.6 | 0.38 | 0.35 |
| Nsit Ubium | | 1 | 4.7833 | 7.9000 | 34.00 | L1 | 285.5 | 129.1 | 0.48 |
| | 2 | 4.7833 | 7.9166 | 49.00 | L2 | 429.2 | 194.7 | 1.23 | 0.77 |
| | | | | | L1 | 326.0 | 147.6 | 0.48 | 0.54 |
| | 3 | 4.8167 | 7.9667 | 37.00 | L2 | 519.9 | 236.0 | 1.23 | 1.13 |
| | | | | | L1 | 269.0 | 121.5 | 0.33 | 0.30 |
| | Ibesikpo | 1 | 4.8500 | 7.9667 | 49.00 | L1 | 350.5 | 158.7 | 0.55 |
| 2 | | 4.9000 | 7.9833 | 133.00 | L2 | 621.0 | 282.1 | 1.75 | 1.62 |
| | | | | | L1 | 218.0 | 98.3 | 0.21 | 0.20 |
| 3 | | 4.9500 | 7.9667 | 72.00 | L2 | 518.9 | 235.6 | 1.22 | 1.13 |
| | | | | | L1 | 325.0 | 147.1 | 0.48 | 0.44 |
| Uruan | | 1 | 4.9167 | 8.0167 | 52.00 | L1 | 295.0 | 133.4 | 0.39 |
| | 2 | 4.9167 | 8.0333 | 52.00 | L2 | 653.1 | 296.8 | 1.94 | 1.79 |
| | | | | | L1 | 334.5 | 151.4 | 0.50 | 0.47 |
| | 3 | 4.9500 | 8.0000 | 57.00 | L2 | 749.3 | 340.7 | 2.55 | 2.36 |
| | | | | | L1 | 306.5 | 138.7 | 0.42 | 0.39 |
| | Nsit Atai | 1 | 4.8667 | 8.0500 | 45.00 | L1 | 313.0 | 141.6 | 0.44 |
| 2 | | 4.8000 | 8.0667 | 37.00 | L2 | 509.4 | 231.2 | 1.18 | 1.09 |
| | | | | | L1 | 313.0 | 141.6 | 0.44 | 0.41 |
| 3 | | 4.8333 | 8.0333 | 31.00 | L2 | 558.4 | 253.6 | 1.41 | 1.31 |
| | | | | | L1 | 302.5 | 136.8 | 0.41 | 0.38 |
| Uyo | | 1 | 4.9833 | 8.0000 | 50.00 | L1 | 302.5 | 136.8 | 0.41 |
| | 2 | 5.0000 | 7.9500 | 82.00 | L2 | 501.3 | 227.5 | 1.14 | 1.05 |
| | | | | | L1 | 303.0 | 137.1 | 0.41 | 0.38 |
| | 3 | 5.0333 | 7.9167 | 67.00 | L2 | 464.7 | 210.9 | 0.98 | 0.91 |
| | | | | | L1 | 372.5 | 168.8 | 0.63 | 0.58 |
| | Itu | 1 | 5.0500 | 7.9167 | 65.00 | L1 | 372.5 | 168.8 | 0.63 |
| 2 | | 5.0667 | 7.9167 | 68.00 | L2 | 558.3 | 253.6 | 1.41 | 1.31 |
| | | | | | L1 | 341.0 | 154.4 | 0.52 | 0.49 |
| 3 | | 5.1000 | 7.9500 | 57.00 | L2 | 533.8 | 242.4 | 1.29 | 1.20 |
| | | | | | L1 | 319.5 | 144.6 | 0.46 | 0.43 |
| Ibiono Ibom | | 1 | 5.1833 | 7.9000 | 66.00 | L1 | 272.5 | 123.1 | 0.33 |
| | 2 | 5.1832 | 7.8667 | 63.00 | L2 | 569.9 | 258.9 | 1.47 | 1.36 |
| | | | | | L1 | 301.0 | 136.1 | 0.41 | 0.38 |
| | 3 | 5.2000 | 7.8500 | 72.00 | L2 | 538.4 | 244.5 | 1.31 | 1.22 |
| | | | | | L1 | 268.5 | 121.3 | 0.32 | 0.30 |
| | 1 | 5.1833 | 7.9000 | 66.00 | L2 | 614.7 | 279.3 | 1.72 | 1.59 |
| L1 | | | | | 237.5 | 107.2 | 0.25 | 0.24 | |
| 2 | 5.1832 | 7.8667 | 63.00 | L2 | 416.5 | 188.9 | 0.78 | 0.73 | |
| | | | | L1 | 278.5 | 125.9 | 0.35 | 0.32 | |
| 3 | 5.2000 | 7.8500 | 72.00 | L2 | 485.2 | 220.2 | 1.07 | 0.99 | |
| | | | | L1 | 331.0 | 149.8 | 0.49 | 0.46 | |
| | | | | | L2 | 492.2 | 223.4 | 1.10 | 1.02 |

The incessant failures of road in the study area can be attributed to the poor compaction of the foundation layers during construction and not due to the geomaterials as the

materials are elastic and can withstand pressure under load. This study identifies that the subgrades in this locality have reliable bearing pressure/capacity to carry the heavy duty

equipment witnessed especially in the urban region of the study area if and only if proper compaction has taken place during road design. Generally, higher values of Lamé's constants are obtained in the southern part of the study area in layer 1. This suggests that the northern parts and other locations need more compaction than the southern part. In

layer 2, the counties within the region have higher values of Lamé's constants while other parts in the north have relatively lower values of Lamé's constants, which in this case are still considered to support good engineering foundation when sufficiently and evenly compacted

Table 2: The minimum, maximum and mean values of Lamé's parameters

| Layer | $\mu \times 10^8 \text{ (N/m}^2\text{)}$ | | | $\lambda \times 10^8 \text{ (N/m}^2\text{)}$ | | |
|-------|--|---------|------|--|---------|------|
| | Minimum | Maximum | Mean | Minimum | Maximum | Mean |
| L1 | 0.21 | 0.63 | 0.43 | 0.20 | 0.58 | 0.40 |
| L2 | 0.78 | 2.55 | 1.30 | 0.73 | 2.36 | 1.30 |

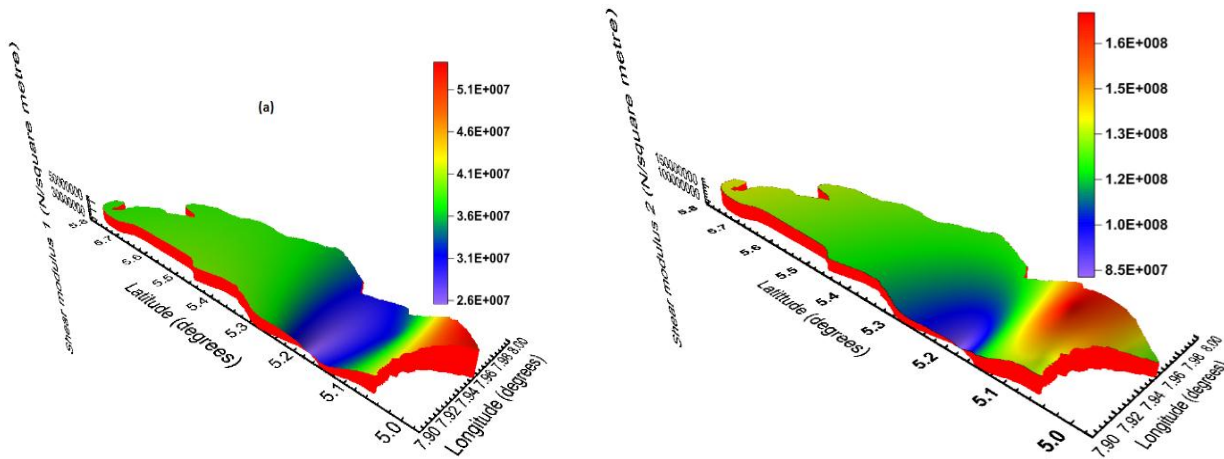


Figure 2: 3-D blanked contour map of (a) layer 1 shear modulus and (b) layer 2 shear modulus in the study area

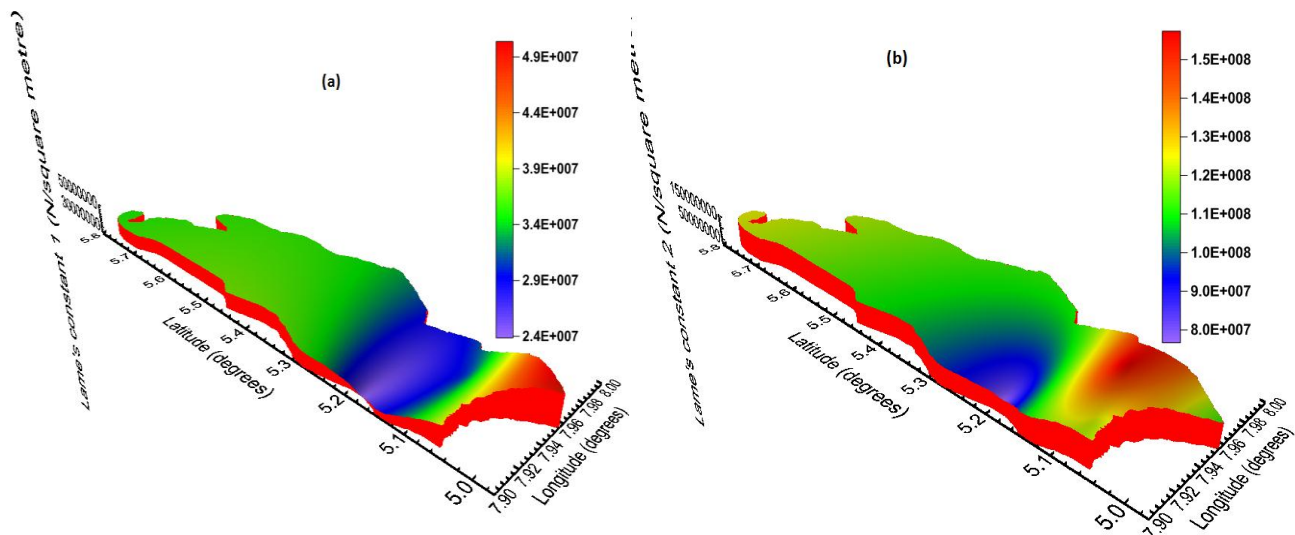


Figure 3: 3-D blanked contour map of (a) layer 1 Lamé's second constant and (b) layer 2 Lamé's second constant in the study area

CONCLUSION

The Lamé's parameters (λ and μ) are two material-dependent quantities in continuum mechanics which arise in stress-strain relationships. Compressional and shear wave velocity

values were estimated for the determination of Lamé's parameters (λ and μ) for Uyo and environ. The values of Lamé's constants (μ, λ) in this work testify that the bearing pressure of the foundation layers is nearly isotropic and

consequently not susceptible to deformation. Lamé's first constant and shear modulus (the second lamé's constant) are linearly related in this study. The geoelastic parameters of the engineering foundation fall within dense sands and gravels as well as silty sands. Soils with high arenaceous formations have a higher bearing capacity than soil with high argillaceous materials.

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