



## APPLICATION OF MULTILINEAR REGRESSION ANALYSIS IN MODELING OF SOIL PROPERTIES FOR GEOTECHNICAL CIVIL ENGINEERING WORKS IN CALABAR SOUTH

J. G. Egbe<sup>1</sup>, D. E. Ewa<sup>2</sup>, S. E. Ubi<sup>3</sup>, G. B. Ikwa<sup>4</sup> and O. O. Tumenayo<sup>5</sup>

<sup>1,2,3,4</sup>, DEPT. OF CIVIL ENGINEERING, CROSS RIVER UNIV. OF TECHNOLOGY, CALABAR, CROSS RIVER STATE, NIGERIA

<sup>5</sup>, DEPT. OF COMPUTER SCIENCE, CROSS RIVER UNIV. OF TECHNOLOGY, CALABAR, CROSS RIVER STATE, NIGERIA

*E-mail addresses:* <sup>1</sup> [jeromelina2011@yahoo.com](mailto:jeromelina2011@yahoo.com), <sup>2</sup> [desmon4sky@gmail.com](mailto:desmon4sky@gmail.com), <sup>3</sup> [emmaubi20@yahoo.com](mailto:emmaubi20@yahoo.com),

<sup>4</sup> [gbite29@yahoo.com](mailto:gbite29@yahoo.com), <sup>5</sup> [ofuiru4u@yahoo.com](mailto:ofuiru4u@yahoo.com)

### ABSTRACT

*The application of Multi-Linear Regression Analysis (MLRA) model for predicting soil properties in Calabar South offers a technical guide and solution in foundation designs problems in the area. Forty-five soil samples were collected from fifteen different boreholes at a different depth and 270 tests were carried out for CBR, MC, SG, LL, PL test and GS with mechanical sieve analysis of sizes, 2.36 mm, 1.18 mm, 600  $\mu$ , 425  $\mu$ , 300 $\mu$ , 212 $\mu$ , 150 $\mu$ , and 75 $\mu$ . This study uses Multi Linear Regression Analysis to formulate a model that relates CBR to other soil parameters. The Multi Linear Regression Analysis was done using the Data Analysis tool of Microsoft Excel, which gave a good coefficient of correlation  $R^2$ , 0.9454 implying that the model can be used to predict CBR at 50.9 % and with  $\pm 3.4\%$  error. This conclusion is drawn since the value of  $R^2$  increases with increase in numbers of variables.*

*Keywords:* Calabar, CBR, Model, Regression, Soil properties, Soil Samples

### 1. INTRODUCTION

The pattern and manner at which structural buildings are collapsing in Nigeria, pose a serious threat to structural engineers, building industry, government, estate developers, building consultants and other relevant stakeholders in the building industry, as well as landlords and tenants. There have been frequent media reports of the collapse of buildings in major cities in Nigeria as a result of poor geotechnical soil investigation and analysis. In many cities in Nigeria have high rising structures which generally, require a factor of safety with respect to different building materials, due to the unpredictability in the analysis of soil and its cost implication.

Many technique for carrying out regression analysis have been developed. Several scholars have reviewed many kinds of literature on the methods of linear regression, ordinary least square regression as parametric, in order hand nonparametric regression is a techniques that permits the regression function to position in a unified set of functions.

Accordingly [1, 3] in their work, they applied linear regression models with more than one independent variable as multiple linear regression and similarly, [1, - 3] in their research they pointed out that MLRA is a well-known method that depicts the relationship that

exists between a set of dependent and independent variables using statistical approaches. Soil forms the basis of support for all engineering structures such as buildings, roadways, dams, importantly many scholars have classified soil properties into three (3) – physical properties (example, colour, porosity, structure, texture); chemical properties (example PH, salinity, cation exchange capacity (CEC), organic matter, C: N ratio (carbon, nitrogen ratio) and mechanical properties such as bearing capacity, permeability, seepage, shear strength, lateral earth pressure etc. Indebt efforts and techniques have been employed in the studies of soil properties, one of such is the use of models [5, 6, 7].

### 2. LITERATURE REVIEW

Also, according to [1, 2, 7], multiple linear regression is one of the modeling technique to investigate the relationship between a dependent variable and several independent variables. It is a generalisation of the simple linear regression model. They stated further that in multiple linear regression models, an error term is assumed to be normally distributed with mean and variance (which is a constant). In the same way, [9 – 12], discussed the risk in Civil engineering, the soil mechanics, and foundation engineering, to identify the

soil properties adsorption with statistical analysis and regression step-by-step using Microsoft Excel. The comparison of artificial neural network with multiple linear regression in prediction soil pore size distribution was examined by [13, 14] they also mentioned multiple-linear regression models for estimating microbial load in a drinking water source. Multiple-linear regression is a predictive analysis approach of representing data and relationship that exists between one dependent and two or more independent variables that use statistic solutions. Similarly, [8, 7, 6], discussed extensively, the generic format of regression models as an error. The model creates a relationship in the form of a straight line (linear) that best approximates all the individual data points. However, multiple linear regression analysis (MLRA) is a statistical technique that uses several explanatory variables to predict the outcome of a response variable. The goal of multiple linear regression (MLR) is to model the relationship between the explanatory and response variables.

### 2.1 Application of MLRA in Modelling Soil Properties

Furthermore, [3, 13] 'introduced' Multiple Regression (MR) analysis and MATLAB programming to estimate the required subsoil properties from the measured properties. They developed a model which allows the decision maker to utilise the available data to achieve possible values of soil properties of non-visited locations. Accordingly, [13] compared MRLA and Artificial Neural Network (ANN) while developing a pedotransfer function for predicting soil parameters. In the work of [5,4,13], they illustrated that, irrespective of the fact that ANN method has some difficulties including various components test of the network structure such as the rule of learning, transfer functions used in the hidden and output layers, number of hidden layers and number of neurons in the hidden layer in trial and error method. [7], discussed the use of multiple linear regression analysis as one of the modeling techniques to depict relationships between microbial raw water quality and physicochemical properties by fitting a linear equation to the observed data set. He published that in order to use the proposed multiple regression analysis, it is necessary to test and verify that the proposed equation satisfies the assumptions.

### 2.2 Limitations of MLRA

Correspondingly, [12] pointed out in his work that the major conceptual constraints of all regression approaches are that only relationships can be achieved, while the underlying causal mechanism can never be

achieved with optimism. Comparably, [12] maintained that MRLA cannot explicitly interpret the multi-collinearity between controlled variables. However, in order to check this, they adopted bivariate correlation analysis and path analysis before setting up a stepwise multi-linear regression. They expressly stated that the problem of multi-collinearity among soil properties in the linear model has been generally recognised in many studies. In the case, of multiple linear regression, this problem can be dealt with by calculating confidence intervals for each partial regression coefficient in the model. However, a more dangerous problem that arises in multiple linear regression is the collinearity or multi-collinearity.

## 3. MATERIALS AND METHODS

### 3.1 Soil Sample Collection

The soil samples (disturbed) for this research were collected from a different location in Calabar South, of Cross River State. It has an area of 264 km<sup>2</sup> and a population of 191,630 based on 2006 Census. Calabar South is located at Latitude: 4° 51' 50.9" (4.8641°) North, Longitude: 8° 20' 10.9" (8.3364°) East and the average elevation of 1 metre. Forty -five (45) soil samples were collected from fifteen (15) different boreholes at a different depth. In each borehole, three sets of representative samples were collected at the depth of 0.5 m, 1 m and 1.5 m respectively. Representative samples were collected using a spade, hand auger, trowel and wrapped in cellophane paper to prevent loss of moisture. The details of the sampling locations are presented in Table 1.

### 3.2 Laboratory Analysis

Wide varieties of laboratory tests were performed on the soils to measure the variation in soil properties. Six different tests were performed on each of the 45 samples. Therefore, in total, 270 laboratory tests were carried out for CBR, Moisture content, Specific gravity, Liquid limit, Plastic limit test and Grain size with mechanical sieve analysis of sizes, 2.36mm, 1.18mm, 600µ, 425µ, 300µ, 212µ, 150µ, 75µ respectively.

### 3.3 Regression Analysis Using Microsoft Excel

Data were arranged in columns in the inbuilt Microsoft excel environment using Excel Data Analysis program the regression data and output were specified.

### 3.4 Formulation of Multi Linear Regression Analysis (MLRA)

The model for MLRA is:

$$Y = b_0 + b_1X_1 + b_2X_2 + \dots + b_nX_n \quad (1)$$

Table 1: Details of sampling locations

| Description of sampling locations | Reference number /Coordinates      |
|-----------------------------------|------------------------------------|
| CRUTECH Quarters                  | BH1, BH2, BH3 4°57'8" N; 8°20'0" E |
| Mount Zion Region                 | BH4, BH5, BH6 4°55' 2"N; 8°21'2"E  |
| Goldie/ Bateba Region             | BH7, BH8, BH9 4°54'4"N; 8°22'8"E   |
| Mbukpa Axis                       | BH10,BH11,BH12 4°58'3"N; 8°22'4"E  |
| Hawkins/ Garden Region            | BH13,BH14,BH15 4°56'10"N;8°24' 8"E |

Where, Y :( CBR %),  $b_1, b_2, b_3... b_n$ : Constants,  $X_1, X_2, X_3 ... X_n$ : Represent the soil properties. To develop the model of multi-linear regression analysis, CBR value is considered as independent variable and soil properties such as natural optimum moisture content (OMC), liquid limit (LL), plastic limit, (PL), maximum dry density (MDD), coarse sand (CS), medium sand (MS), and fine sand (FS) are considered as the dependent variables. For this work, MLRA is carried out using the data analysis toolbar of Microsoft Excel in other to derive the relationship statistically, see Table 2 and 3 and the regression model of the soil properties in Table 4.

Table 2: Descriptive Statistics for the Soil Properties

| Variables | N  | Mean  | Median | Tr Mean | StDev | SE Mean |
|-----------|----|-------|--------|---------|-------|---------|
| CBR (%)   | 15 | 6.967 | 7.800  | 7.192   | 2.510 | 0.648   |
| OMC (%)   | 15 | 14.86 | 14.06  | 14.63   | 4.73  | 1.22    |
| LL (%)    | 15 | 44.30 | 45.50  | 44.65   | 7.76  | 2.00    |
| PL (%)    | 15 | 39.25 | 39.13  | 39.19   | 4.49  | 1.16    |
| MDD (%)   | 15 | 4.795 | 4.330  | 4.315   | 2.927 | 0.756   |
| CS (%)    | 15 | 13.03 | 6.10   | 11.091  | 14.37 | 3.71    |
| MS (%)    | 15 | 18.95 | 11.50  | 17.68   | 15.61 | 4.03    |
| FS (%)    | 15 | 12.69 | 6.30   | 10.37   | 15.88 | 4.10    |

Table 3: Minimum, Maximum and coefficient of the soil properties

| Variables | Min   | Max    | Q <sub>1</sub> | Q <sub>2</sub> |
|-----------|-------|--------|----------------|----------------|
| CBR       | 1.100 | 9.900  | 6.900          | 8.400          |
| OMC       | 7.48  | 25.14  | 11.79          | 17.86          |
| LL        | 28.00 | 56.00  | 40.00          | 50.00          |
| PL        | 31.27 | 48.00  | 36.80          | 41.17          |
| MDD       | 1.500 | 14.330 | 3.500          | 4.830          |
| CS        | 0.50  | 50.70  | 2.00           | 21.00          |
| MS        | 1.90  | 52.50  | 6.50           | 30.00          |
| FS        | 0.48  | 55.10  | 1.50           | 28.00          |

Table 4: Predictor coefficient

| Predictor | Coefficient | StDev   | T     | P     |
|-----------|-------------|---------|-------|-------|
| Constant  | 20.774      | 7.913   | 2.63  | 0.034 |
| OMC       | -0.1018     | 0.1920  | -0.53 | 0.612 |
| LL        | -0.1616     | 0.2028  | -0.80 | 0.452 |
| PL        | -0.1204     | 0.2381  | -0.51 | 0.628 |
| MDD       | 0.1446      | 0.3262  | 0.44  | 0.671 |
| CS        | -0.0350     | 0.06562 | -0.05 | 0.959 |
| MS        | 0.02464     | 0.05468 | 0.45  | 0.666 |
| FS        | -0.12004    | 0.08733 | -1.37 | 0.212 |

Table 5: Regression equations of CBR for different soil parameters

| Description | Regression Model |
|-------------|------------------|
| CBR VS MC   | 6.69 +0.018 MC   |
| CBR VS LL   | 8.33 -0.0307LL   |
| CBR VS PL   | 17.6 -0.272PL    |
| CBR VS MDD  | 6.08 +0.198 MDD  |
| CBR VS CS   | 6.08 +0.0682 CS  |
| CBR VS MS   | 6.53 +0.0232 MS  |
| CBR VS FS   | 7.79 -0.0647 FS  |

CBR = 20.8 - 0.102MC- 0.162 LL-0.120 PL + 0.145 MDD -0.0035CS + 0.0246MS - 0.120FS (2)  
Therefore, equation (2) represent the multi-linear equation that relates CBR to other soil properties in Calabar South the value of CBR as coefficient was derived from Table 3.

4. RESULTS AND DISCUSSION

The raw results obtained from the laboratory experiments are shown in Table 6 and Figures 1 to 8. The analyses of soil, compaction characteristics and CBR (%) of different soil samples, the results showed that MDD has values of 4.66, 4.33, 2.66, 4.83, 14.33, 4.0, 3.5, 4.33,4.66, 4.0, 1.5, 2.33, 6.8 and 5.16 for A-1-b, A-3, A-5, A-5-7 and A-2-6 based on AASHTO classification system, see Table 6.

Similarly, the optimum moisture content (OMC) the values were, 17.86,16.9, 14.06, 13.46, 25.14, 13.3,11.79, 13.85,20.69,19.73,9.43, 8.78,15.63,7.48 and 14.73 for A-1-b, A-3, A-5, A-5-7 and A-2-6 respectively. The AASHTO system classifies soils into seven primary groups, namely A-1 through A-7, based on their relative expected quality for road embankments, subgrades, sub-bases, and bases. Some of the groups are further divided into subgroups, such as A-1-a and A-1-b as shown in Table 6. Basically, there are two soil classification systems often use for engineering purposes.

The Unified Soil Classification System (USCS) which is basically, used for virtually all geotechnical engineering work except road construction and highway, but the AASHTO soil classification system is used for both. The sample BH1, BH8, BH9, B10, and BH13 are purely clay soil for A-1-b, BH2 and BH6 are silt soil for A-3, A-2-5, BH3 and BH14 are clayey silt soil for A-3, BH4 is

clay silt soil for A-3, BH5 and BH15 are clayey soil for A-2-6, A-1-b, BH7 is sandy silt for A-5 and BH11 is silty sand for A-2-5 respectively.

Clays are compressible, for instance, if a moist mass of clay is subjected to compression, moisture and air may be expelled, resulting in a reduction in volume which is not immediately recovered when the compression load is withdrawn. The reduction in volume per unit increase of pressure is defined as the compressibility of soil, and a measure of the rate at which consolidation proceeds is given by the 'co-efficient of consolidation' of the soil.

Silts are the nonplastic fines, they are inherently unstable in the presence of water and have a tendency to become "quick" when saturated that is, they assume

the character of a viscous fluid and can flow. Silts are fairly impervious, difficult to compact, and highly susceptible to frost heaving.

**4.2 Correlation between CBR (%) and different soil types**

There was a strong significant positive correlation between CBR and OMC and this indicates that as OMC increases CBR value decreases ( $R^2 = 0.1$ ). The correlation between CBR and different soil types are presented in Table 6. The correlation between LL, PL, and CBR depict that as the values of LL, PL increases the value of CBR decreases indicating that there was a weak positive non-significant correlation.

Table 6: Soil classification of the location

| SAMPLE NO. | CBR % | MDD (gcm <sup>3</sup> ) | RANGE of OMC % | USCS         | AASHTO Classification |
|------------|-------|-------------------------|----------------|--------------|-----------------------|
| BH1        | 7.8   | 4.66                    | 17.86          | Clay         | A-1-b                 |
| BH2        | 9.9   | 4.33                    | 16.90          | Silt         | A-3                   |
| BH3        | 7.1   | 2.66                    | 14.06          | Clayey Silt  | A-3                   |
| BH4        | 7.9   | 4.83                    | 13.46          | Clay Silt    | A-3                   |
| BH5        | 7.5   | 14.33                   | 25.14          | Clayey       | A-1-b                 |
| BH6        | 8.4   | 4.00                    | 13.30          | Silt         | A-2-5                 |
| BH7        | 7.2   | 3.50                    | 11.79          | Sandy Silt   | A-5                   |
| BH8        | 6.9   | 4.33                    | 13.85          | Clay         | A-3                   |
| BH9        | 8.6   | 4.66                    | 20.69          | Clay         | A-5-7                 |
| BH10       | 1.1   | 4.00                    | 19.43          | Clay         | A-5-7                 |
| BH11       | 1.2   | 1.50                    | 9.43           | Silty Sand   | A-2-5                 |
| BH12       | 8.5   | 2.33                    | 8.78           | Silty Clayey | A-3                   |
| BH13       | 6.3   | 4.83                    | 15.63          | Clay         | A-3                   |
| BH14       | 7.8   | 6.8                     | 7.48           | Clayey Silty | A-3                   |
| BH15       | 8.3   | 5.16                    | 14.73          | Clayey       | A-2-6                 |

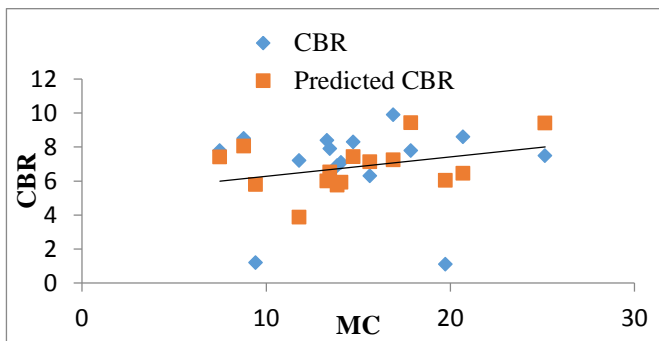


Figure 1: Relationship between CBR and MC

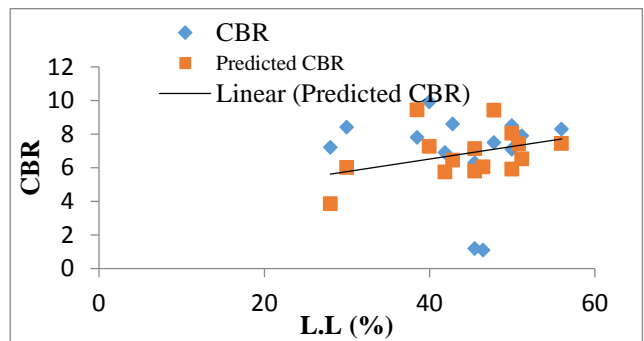


Figure 2: Relationship between CBR and LL

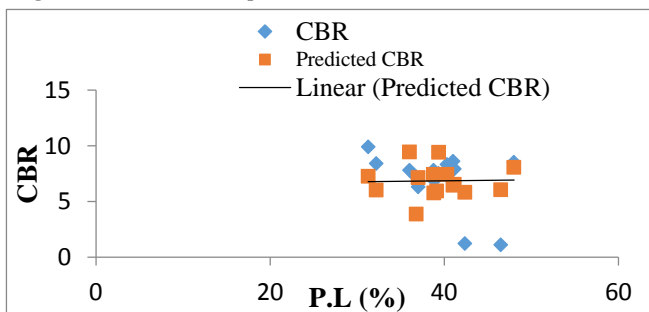


Figure 3: Relationship between CBR and PL

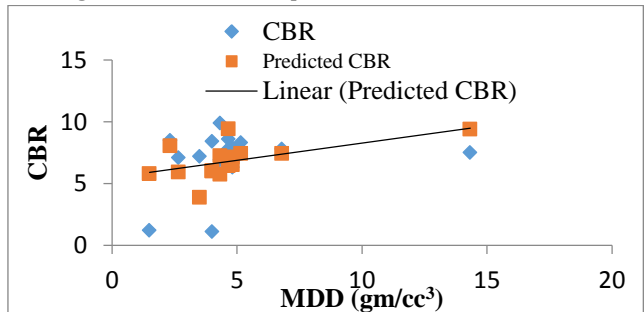


Figure 4: Relationship between CBR and MDD

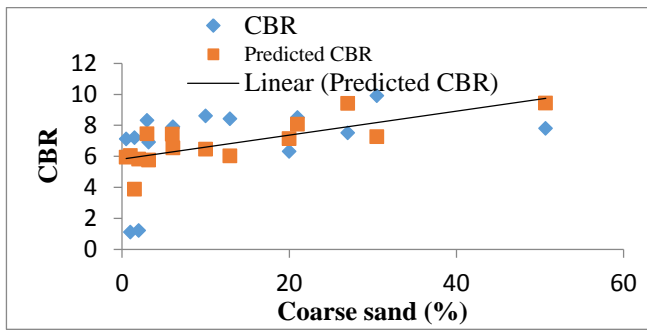


Figure 5: Relationship between CBR and CS

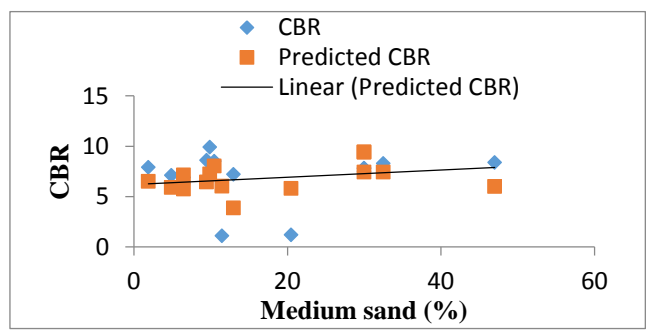


Figure 6: Relationship between CBR and MS

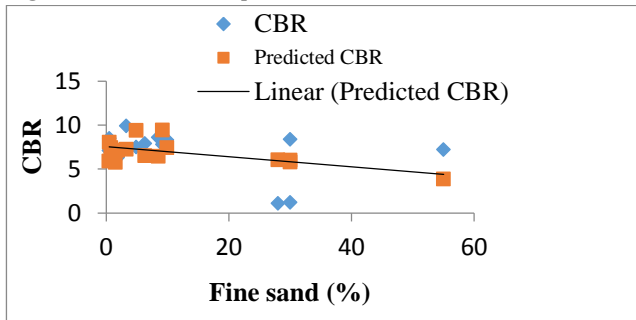


Figure 7: Relationship between CBR and FS parameters

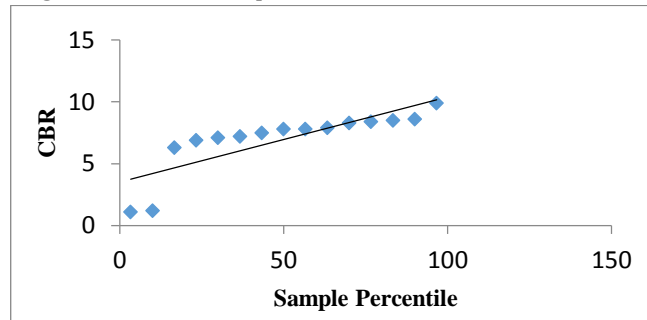


Figure 8: Relationship between CBR and 7 other soil properties

These results confirmed partly with [15], and [16], who reported that plastic limit had a strong correlation with CBR. Closely, liquid limit (LL) and plastic limit (PL) are two important states of consistency. Liquid limit is the percentage moisture content at which a soil changes with decreasing wetness from the liquid to the plastic stability or with increasing wetness from the plastic to the liquid consistency, whereas the plastic limit is the percentage moisture content at which a soil changes with decreasing wetness from the plastic to the semi-solid consistency or from the semi-solid to the plastic consistency. The compaction characteristics and correlation between CBR and MDD in Tables 6 and 8, show that as the values of CBR increases the MDD values decreases showing positive correlation value ( $R^2= 5.3\%$ ). Similarly, the correlations between CBR, MS and CS indicates that as the values of CBR decreases there is a corresponding increase in MS and CS values showing a positive correlation value respectively. See Table 8.

This implies that the model in the equation 1, can predict CBR from the considered parameters at 50.9% with plus or minus 3.4% error. The adjusted R-square, a measure of explanatory power, is 0.6891. The multiple correlation coefficients (multiple R) is 0.9454. These indicate that the correlation among the independent and dependent variables are positive. Figures 9-12 clearly, show the comparison between actual values, forecast values and exponential smoothing of CBR against others parameters respectively.

**5. CONCLUSION**

From the above findings, it can be concluded that the application of Multi-Linear Regression Analysis (MLRA) model for predicting soil properties in Calabar South offers a technical guide and solution in foundation designs problems in the area.

Table 7: Correlation between CBR (%) and different soil properties

|     | CBR    | MC      | LL     | PL      | MDD     | CS      | MS    |
|-----|--------|---------|--------|---------|---------|---------|-------|
| MC  | 0.035  |         |        |         |         |         |       |
| LL  | -0.095 | - 0.034 |        |         |         |         |       |
| PL  | -0.486 | - 0.105 | 0.577  |         |         |         |       |
| MDD | 0.231  | 0.605   | 0.175  | - 0.130 |         |         |       |
| CS  | 0.391  | 0.369   | -0.189 | - 0.352 | 0.298   |         |       |
| MS  | 0.144  | 0.006   | -0.034 | - 0.258 | 0.290   | 0.136   |       |
| FS  | -0.409 | - 0.120 | -0.651 | - 0.068 | - 0.262 | - 0.316 | 0.167 |

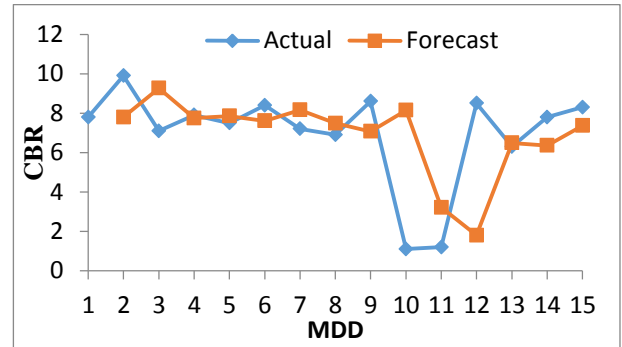
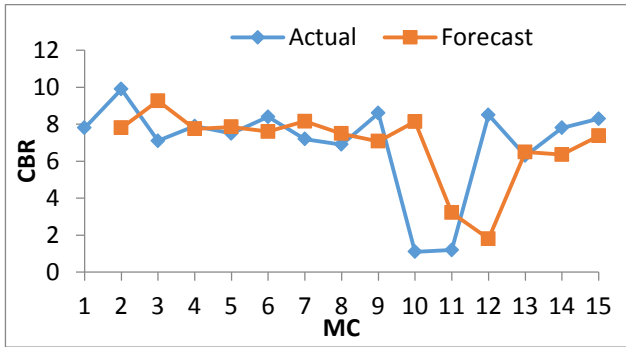


Figure 9: Comparison between actual values and forecast values of CBR for MC and MDD

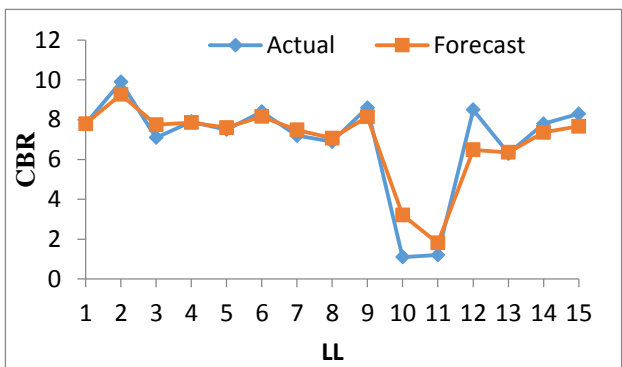
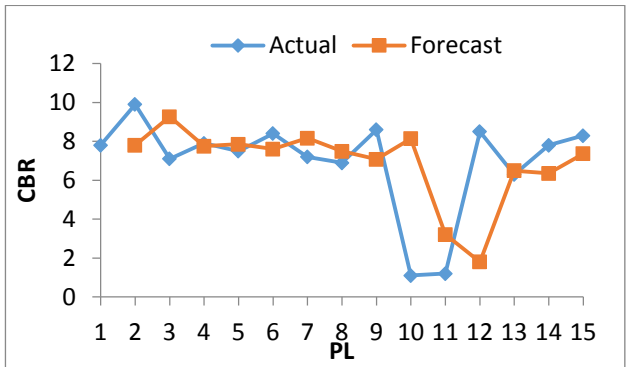
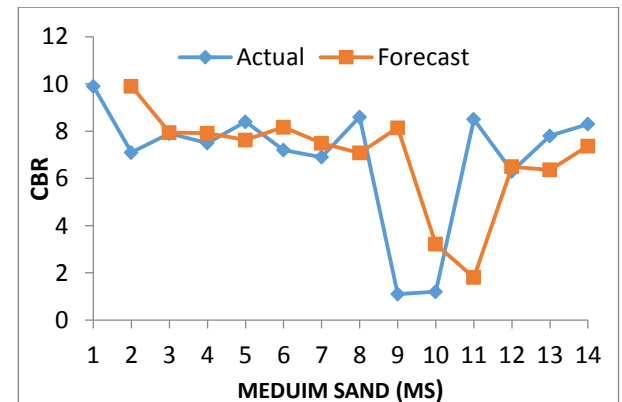
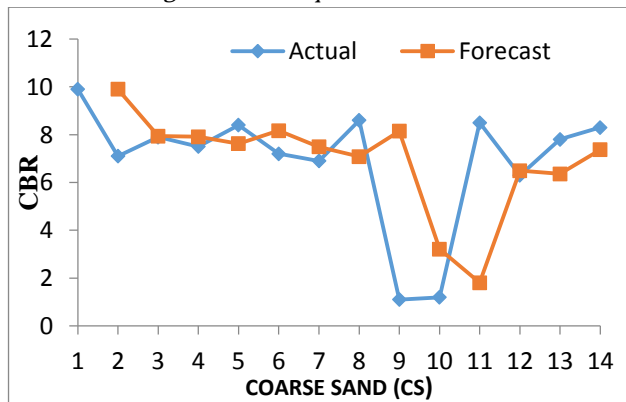


Figure 10: Comparison between actual values and forecast values of CBR for PL and LL



Figures 11: Comparison between Actual values and forecast values CBR CS and MS

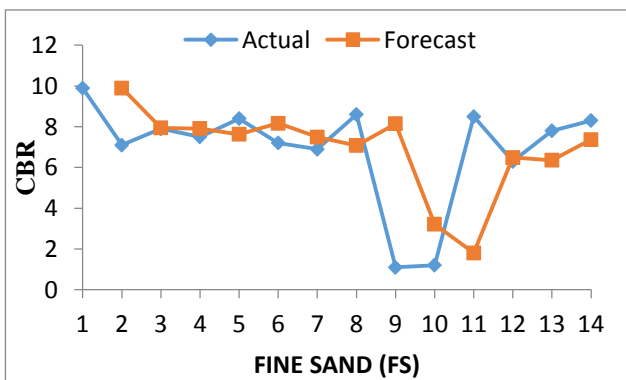


Figure 12: Comparison between actual value and forecast value of CBR for FS

It is clearly, established that maximum dry density (MDD) and optimum moisture content (OMC) affects directly the CBR value for different soil types. This

shows that the CBR value prediction model based on the MLRA are close to experimental actual values for different soil types and it can predict or forecast the value of CBR in the study area as shown in Figures 9-12. Accurate analyses of soil are important in ensuring that these structures remain stable and safe. The CBR value is an essential soil property for designing of flexible pavement and runway of airfields and can be used for determining the reaction of the soil subgrade. Considering the constraints of laboratory work in determining the California Bearing Ratio (CBR), the MLRA model is proposed for correlating CBR value with some soil parameters. The result gave a good coefficient of correlation,  $R^2$  as 0.9, implying that the derived MLRA model can be used to predict CBR at 50.9% with  $\pm 3.4\%$  error. Further studies to extend this

research will involve number of samples across the Calabar Metropolis will be developed to study the correlation of CBR with different types of soils to give adequate and accurate recommendation.

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