

EFFECTS OF WEATHERING ON MECHANICAL PROPERTIES OF GRANITE GNEISS AND DOLERITE: A CASE STUDY OF OBAN AND OBUDU BASEMENT ROCKS SOUTH-EASTERN NIGERIA

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ABSTRACT

This paper describes the geotechnical characteristics of granite gneiss and dolerite from Oban and Obudu regions in South-eastern Nigeria. The suitability of these rocks for civil construction purposes and the effects of weathering on their mechanical properties are examined. The results indicate that compressive strength ranges from 19.31 to 77.37 MN/m² and 40.5 to 140.0 MN/m² for the granite gneiss and dolerite respectively. Their respective Young Modulus ranged 13.60 to 84.20 GPa and 24.45 to 102.50 GPa. The low values obtained for compressive strength and Young Modulus in this work are the direct effects of the humid tropical conditions that cause weathering of the rocks at the base of the overburden aided by water. In addition to weathering, structural defects such as fractures, joints, faults and the mechanical drilling process tend to introduce micro-pores/fractures in the samples thereby leading to a decrease in the strength and modulus of these rocks. Furthermore, the moisture content (W%) and the degree of weathering decrease with increasing hardness (H), specific gravity (SG), compressive strength (α) and Young Modulus (E). The relationship between moisture content, the degree of weathering and the mechanical properties – particularly compressive strength (α) and Young modulus (E) also shows that the fresh (FR) to slightly weathered (SW) rocks are generally suitable for most civil construction works than the extremely weathered (EW) to highly weathered (HW) rocks.

KEYWORDS: Basement Rocks, Mechanical Properties, Weathering Grades (FR-EW), Oban/Obudu.

INTRODUCTION

Basement rocks are known to occur at Oban and Obudu regions of South-eastern Nigeria (Ekwueme 1988, 1990, 2003). These rocks when quarried could provide raw materials for highway pavement aggregates, civil construction works, rip rap for dams and

monumental stones at affordable prices. The Oban massif occupies an area of about 10,000 km² (Ekwueme et. al., 1995) and lies between Longitudes (8°00' and 8°55' E) and latitudes (5°00' and 5°45') while Obudu plateau extends over an area of about 4500 km² between latitudes 6° 00' N and 6° 45' N and longitudes 8° 40' E and 9° 30' E (Fig.1).

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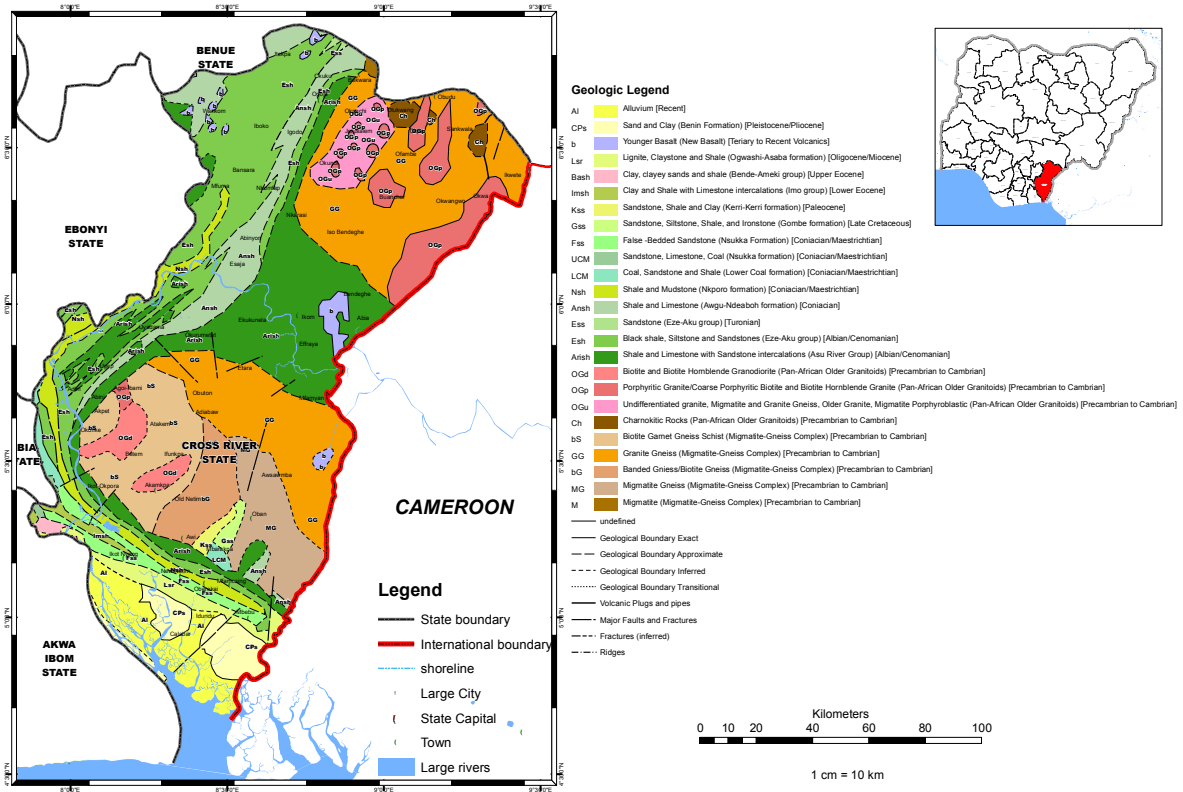


Fig 1. Geological Map of Cross River State

The climate is tropical. There are distinct wet and dry seasons, the former from April to October and the latter from November to March. The areas are underlain by highly deformed Precambrian crystalline basement rocks mainly granite, gneisses and schists. These rocks are intruded by pegmatites, granodiorites, charnokites and dolerites (Ekwueme 1990, 1998). They are deeply weathered in some places to silty clay (Fig. 2) and could

affect the assessment of overall modulus and strength of rock performance in engineering works (Amah et al. 2012). A typical weathering profile in a basement terrain is illustrated in Fig. 2. Both granite gneiss and dolerite consist of a wide range of weathering grades: Extremely Weathered (EW), Highly Weathered (HW), Moderately Weathered (MW), Slightly Weathered (SW) and Fresh Rock (FR).

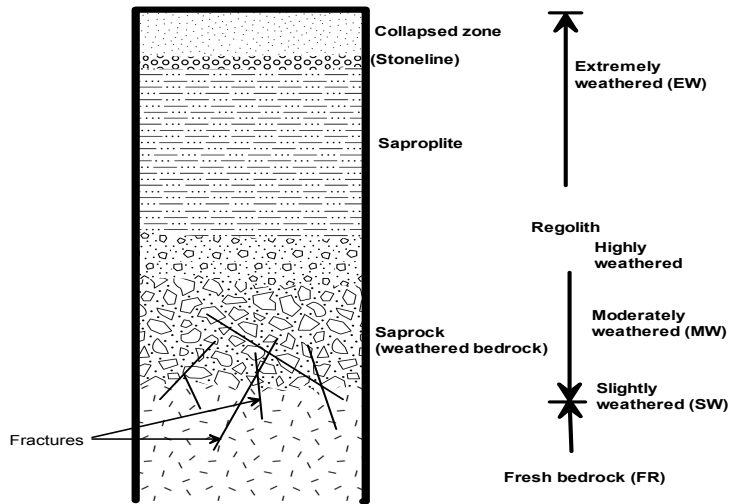


Fig.2: Typical weathering profile in a basement complex terrain(modified from Wright, 1992)

Dolerites are igneous rocks of typically hypabyssal origin having formed as shallow sills and dykes. They may be regarded as equivalents of gabbros of plutonic origin and basalts of volcanic origin. The term diabase is also used for dolerite. They are predominantly made up of calcic plagioclase and are mostly medium to fine grained rocks. Granite gneiss on the other hand, is a megascopically crystalline foliated metamorphic rock characterized by segregation of constituent minerals into layers or bands of micaceous minerals, alternating with bands of equidimensional minerals like feldspars, quartz and garnet etc(Ekwueme 2003).

Granite gneiss and dolerites are quite common in Oban and Obudu basement regions of South-eastern Nigeria. They are extensively used in materials for construction. They are characterized by very high crushing (compressive) strength and hence can be easily trusted in most of construction works. These

rocks are typically impervious, hard and strong and form very strong foundations for most civil engineering projects. Granite gneiss and dolerite, fail when subjected to loads beyond their strength. The failure takes place under compressive, tension and shear forces at different values. It is however, the unconfined compressive strength, which is taken as the most important index property of rocks.

The compressive strength of a rock depends on a number of factors such as its mode of formation, its composition, texture and structure, its moisture content and extent (degree) of weathering it has already suffered.

During the last few years, thousands of tests have been carried out on rock strength in different parts of the world. An attempt has been made by Deere and Miller (1966) to classify rocks on the basis of compressive strength range (Table 1).

Table 1: Engineering classification of rocks by Deere and Miller(1966)

Class	Type description	Uniaxial compressive strength (MN/m ²)
A	Very high strength	> 224.0
B	High strength	112.0 – 224.0
C	Medium strength	56.0 – 112.0
D	Low strength	28.0 – 56.0
E	Very low strength	< 28.0

In the area of study, few of the published works have been on the geology and petrology of Oban/Obudu basement rocks (Rahman et al. 1981, Ekwueme 1990). Different authors have also established the economic importance of basement complex of Nigeria and most importantly, as a good material for civil construction (Rahaman, 1976, Dada 2006, Bale et al. 2010, Amah et al. 2012). The present paper therefore focuses on the physical and mechanical properties of granite gneiss and dolerite, in particular the effect of weathering on strength and modulus of these rocks from Oban massif and Obudu plateau regions.

METHOD OF STUDY

Field work

The bulk of samples for this work were rock cores and powder obtained from drill holes at some of the quarry sites at Oban and Obudu areas. The drill holes locations were accurately surveyed using the Garmin 76 Global Positioning System (GPS) to obtain their latitudes and longitudes as well as the relative elevation data. Each borehole had a maximum depth of 15m and samples collected at 3m depth intervals were placed in polythene bags which were accurately labeled for laboratory testing. Bulk samples were also obtained from rock outcrops in-situ and cut into cylinder for the compressive strength test/Young modulus.

Laboratory tests

The following physical and engineering tests were performed on over 60 rock specimens of granite gneiss and 56 samples of dolerite from some of the quarry sites within the Oban/Obudu regions. These include: Specific gravity (SG), Hardness (H), moisture content (w), compressive strength (α) and Young modulus (E). The specific gravity was determined by using a 50cm³ density bottle. This method was found suitable because the samples were in powder form. The Moh's Hardness (H) is defined as the resistance to scratching by various implements. The scale of Hardness ranges from very soft (grade 1) to very hard (grade 10). Rocks at the sites are composed of mineral aggregates, thus a number of Moh's hardness tests were performed and the average results taken. Moisture content w% (the weight of water in the samples to weight of solids) was carried out as described by ASTM 2216 – 71.

The uniaxial compressive strength and Young modulus of the rock blocks were also determined using the Universal Testing Machine (UTM) model AXM500-50KN. The compressive strength measurements on standard specimens of 50 mm in diameter and 50 mm in length with coplanar end-faces were performed (with an accuracy of 0.1%). The load was applied to the end-faces of the specimen with a strain rate of 1,000 N/s until failure. The maximum load is defined as the uniaxial compressive strength. The Young's modulus was determined from the stress-strain curve of the uniaxial compressive strength test. The results are presented in Table 2.

RESULTS AND DISCUSSION

The results of the physical (H, W%, SG) and mechanical properties (α , E) are shown in Table 2.

Table 2: Summary of physical and mechanical properties of basement rocks (granite gneiss/dolerite) from South-eastern Nigeria

S/N	Properties	Granite gneiss (n=60)	Dolerite (n=56)
1	Specific gravity (SG)	1.55 – 2.73	1.50 – 2.65
2	Hardness (H)	1.0 – 6.0	1.0 – 6.5
3	Moisture content (W)%	0.11 – 10.12	0.10 – 8.40
4	Compressive strength (α) MN/m ²	19.31 – 77.37	40.5 – 140.0
5	Young Modulus (Y) 10 ³ MN/m ²	13.60 – 84.20	24.45 – 102.50

n=number of rock specimens

The specific gravity

The specific gravity of samples tested ranges from 1.55 (overburden) to 2.73 (fresh granite gneiss) and 1.50 to 2.65 for dolerite. SG tends to increase with increasing rock quality or strength. In a case where the SG shows an unexpected decrease with depth, some

weathering of the rocks must have taken place or the fracture might contain water. Fresh rocks at the Oban/Obudu quarry sites generally have SG values greater than 2.40 in both regions while the weathered/overburden depth range usually have SG values less than 2.40.

Moisture content (W%)

The results (Table 2) reveal that the water content ranges from 0.11 to 10.12% in both rocks. There is a general decrease in the water content of the samples with increasing depth. That is, more water is contained in the weathered overburden materials than in the sound rock sections. The implication of the decrease of moisture with depth is that fractures (if any) in the rocks at the site also decrease with depth. Hence, swelling pressure potential due to the present of clay mineral resulting from weathering of the aluminosilicates is very low (Edet 1992)

Moh's Hardness (H)

The hardness H ranges from 1.0 for weathered soft rock to a value slightly above 6.0 for fresh granite gneiss/dolerite. The low Moh's hardness obtained for overburden materials indicate poor sample quality. Increase hardness is directly related to improve rock quality and strength.

Compressive strength (α) and Young modulus(Y)

The results of the compressive strength of rocks samples at the Oban and Obudu are indicated in Table 2. It ranges from 19.31 to 77.37 MN/m² and 40.5 to 140.0 MN/m² for the granite gneiss and dolerite respectively. Rock strength results reveal slightly low values compared to the standard range values of 80.0 to 250.0 MN/m² for fresh metamorphic rocks such as gneiss and 100.0 to 350.0 MN/m² for fresh igneous rocks such as granite and dolerite (Deere and Miller 1966, Singh 2008). Their respective Young Modulus ranged

13.60 to 84.20 GPa and 24.45 to 102.50 GPa. The low values obtained for compressive strength and Young Modulus are direct effects of the humid tropical conditions that causes weathering of the rocks at the base of the overburden aided by water. In addition to weathering, structural defects such as fractures, joints, faults and the mechanical drilling process may tend to introduce micro-pores/fractures in the samples (Wang and Simmons 1978, Kowallis 1982) thereby leading to a decrease in the strength and modulus of these rocks. These might also be responsible for the slightly lower SG values obtained in this work. Generally, the mechanical properties of basement rocks in Obudu area tend to be comparatively higher (better rock quality) than those found in Oban region. Rocks in Oban massif are heavily weathered and of higher fracture density than those of Obudu. This observation could be partly due to the fact that Oban is located in a region with high rainfall/thick vegetation which accelerate chemical weathering in the rocks of the area (Amah et al. 2012), and partly due to the tectonic forces operating during the Pan African orogeny that was assisted by fluids in reworking of basement fractures in Oban massif than that of Obudu basement (Oden et al. 2012)

Correlation diagrams

A plot of unconfined compressive strength (α) versus moisture content (w), (Fig. 3) and Young modulus (E) against moisture content (w), (Fig. 4) and weathering grades (EW to FR) for both dolerites and granite gneiss indicate that the higher the moisture content (w), the higher the weathering grades (FR-EW)

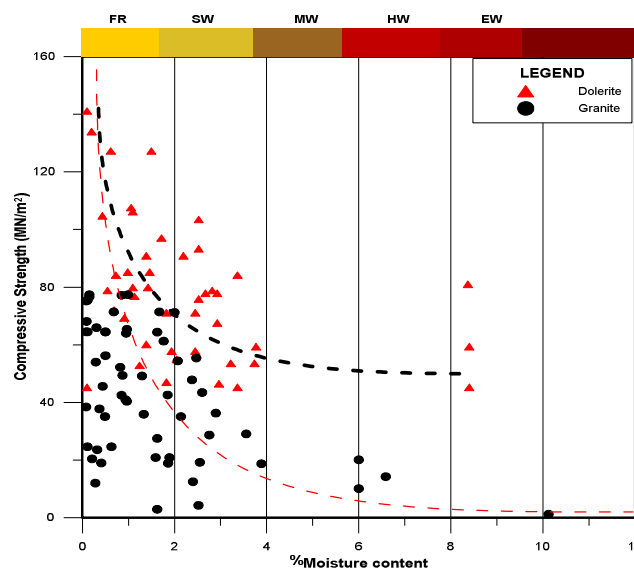


Fig. 3: Compressive Strength vs Moisture Content

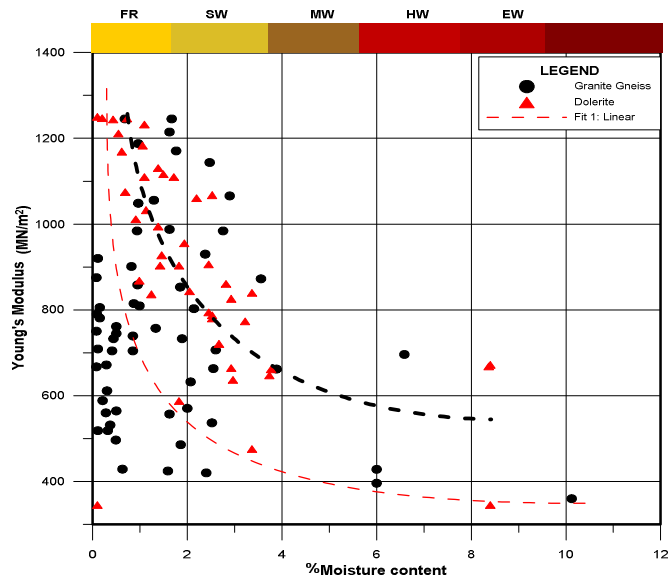


Fig. 4: Young's Modulus vs Moisture content

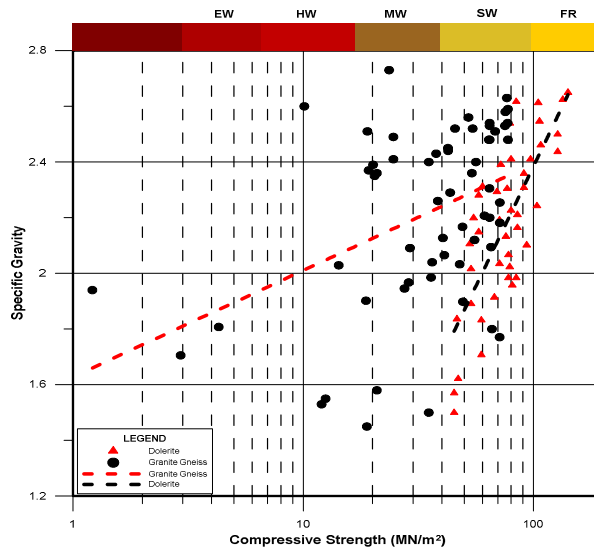


Fig.5: Specific Gravity vs compressive strength

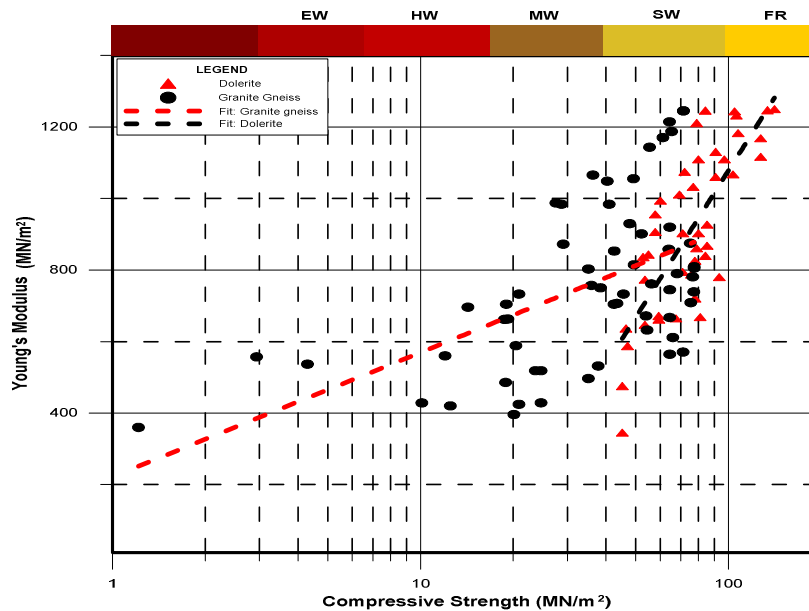
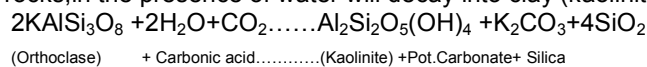


Fig. 6: Young Modulus Vs compressive strength

The diagrams further show a general trend for unconfined compressive strength(α)/YoungModulus(E) decreasing with the increasing moisture content (weathering grades). The data indicates some scatter due to vesicularity of the samples tested as well as inclusion of data biased by joint/fracture influences.

A plot of SG against compressive strength (Fig. 5) and compressive strength versus Young modulus and weathering grades (Fig 6) clearly reveal that the higher the specific gravity SG the higher the compressive strength (or Young Modulus) which implies increasing rock quality with depth and appears to show negative correlation with the degree of weathering.

In-situ identification and field observations have confirmed that granite gneisses are affected by weathering processes faster than the dolerite because of the presence of foliation planes in gneisses and their complete absence in dolerite. Both rocks, in the presence of water will decay into clay (kaolinite) and silica according to the equation below:



(Orthoclase) + Carbonic acid.....(Kaolinite) + Pot. Carbonate + Silica

Furthermore, in the above equation, sodium (Na) or calcium (Ca) may be present instead of potassium (K) if the mineral in question is another type of feldspar. The main end product, Kaolinite is formed in all such cases. This chemical change in the rock produces definite alteration in the physical constitution of the rock: a soft (H = 1) clay mineral is formed in place of a hard mineral (feldspar, H = 6), thereby affecting the strength of the rock very significantly. Carbonates are removed in solution and silica formed colloids; this chemical weathering processes result in partial or total conversion of a strong igneous rock like dolerite and metamorphic rocks such as granite gneiss into a mass of soft clay like product in the zone of weathering because feldspars are their chief constituent minerals (Singh 2008, Amah et. al. 2012).

Classification scheme

The relationship between the degree of weathering (FR-EW), moisture content (W) and mechanical properties particularly compressive strength (α) and Young modulus (E) helps to categorize the rocks into 5 classes A, B, C, D and E (Table 3). The fresh rock (FR) and slightly weathered (SW) rock are classified under classes A and B respectively. These rocks have compressive strength ($>80\text{MN/m}^2$) and ($80-60\text{MN/m}^2$) with very low water content (0-4%). Moderately weathered (MW) rocks have a medium compressive strength ($60-40\text{MN/m}^2$), moderate water content (4-6%) and is classified as class C. Rocks that are highly weathered (HW) and extremely weathered (EW) are put under classes D and E respectively. Class D has low compressive strength ($40-20\text{MN/m}^2$) and high water content (6.0-8%). However, Class E are rocks with low compressive strength $<20\text{MN/m}^2$ and very high moisture content $>8\%$ (Table 3).

Table 3: Rock strength in relation to the end use

Compressive strength (MN/m ²)	Young modulus (GPa)	Moisture contents (%)	Degree of weathering	Classes	End use
< 20.0 (very low)	<10.00 (very low)	> 8 (very high)	EW	E	Unsuitable for embankments foundation, airfield and highway pavement.
20.0-40.0 (low)	10.00-20.00 (low)	6 – 8 (high)	HW	D	Suitable for foundations and embankments.
40.0-60.0 (medium)	20.0-30.0 (medium)	4-6 (medium)	MW	C	Suitable for civil constructions works/airfield pavements
60.0-80.0 (high)	30.0-40.0 (high)	2-4 (low)	SW	B	Very suitable for civil construction works.
>80.0 (very high)	>40.0 (very high)	< 2 (very low)	FR	A	Most suitable for all civil construction works.

EW – Extremely weathered
 HW – Highly Weathered
 MW – Moderately Weathered
 SW – Slightly Weathered
 FR – Fresh Rock

The (FR, SW and MW) rocks are capable of withstanding heavy traffic foundation loads and generally suitable for most civil construction works (Table 3) because of their high strength and almost zero water content (Amah et. al. 2012). 60% of the test results are within these grades (Fig.5 and 6). Rock samples whose crushing strength fall in the range (40-20)MN/m² (class D) are suitable for use in foundations and embankment constructions in spite of their low compressive strength. About 30% of the test results presented fall within this category (Fig.5 and 6). Rock samples with compressive strengths less than 20MN/m² (class E) are considered unsuitable for embankments, foundations, air field and highway pavements. This is due to their high water contents and very low compressive strength. The HW-EW rocks (i.e. classes D and E) are mainly weathered overburden materials which must be removed before engineering loads are placed on them to avoid engineering geological hazards. These overburden materials may not be a waste in all cases. They could be used for filling and reclamation of excavated areas.

CONCLUSION

The basement rocks of Oban and Obudu in South Eastern Nigeria have experienced various degrees of weathering: Extremely Weathered (EW), Highly Weathered (HW), Moderately Weathered (MW), Slightly Weathered (SW), and Fresh Rocks (FR). Based on the degree of weathering, moisture contents, and the mechanical properties particularly (compressive strength and young modulus E) the rocks have been grouped

into classes A, B, C, D and E. The results indicate that the higher the moisture content the higher the degree of weathering and the lower the compressive strength (or Young modulus) of the rocks.

Generally, on the basis of the assessment scheme (Table 3) the fresh rocks (FR) to Slightly weathered (SW) rocks from Oban/Obudu basement grouped into classes A and B show a better rock quality than the highly and extremely weathered overburden (HW-EW) rocks (classes D and E). They are judged adequate for road constructions, airfield pavement, foundations and other civil engineering works, irrespective of the humid tropical weather conditions. Class C is regarded as intermediate or transitional. The overburden materials (HW-EW) must be removed before loads are placed on the foundation or transported to elsewhere to be used for filling and reclamation of excavated areas.

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